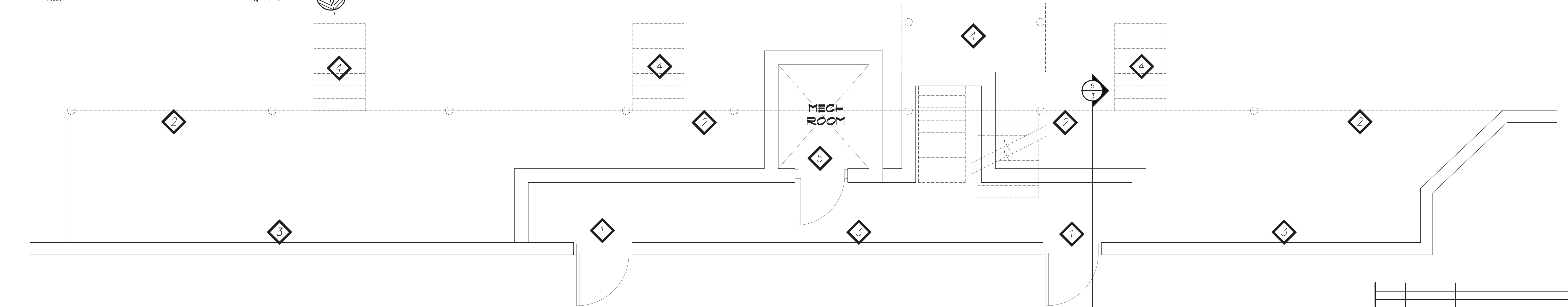


EXISTING TYPICAL DEMOLITION PLAN
SCALE: 3/8" = 1'-0"



EXISTING BASEMENT DEMOLITION PLAN
SCALE: 3/8" = 1'-0"

NOTE

- 1 LOCK (BARRICADE ALL ACCESS DOORS TO BALCONY)
- 2 REMOVE EXISTING BALCONY (3 LEVELS) INCLUDING COLUMNS & FOOTINGS.
- 3 DISCONNECT BALCONY FROM MAIN BUILDING. MAKE GOOD ANY DAMAGE TO BUILDING/SIDING.
- 4 REMOVE EXISTING STAIRS COMPLETELY INCLUDING FOOTINGS
- 5 ELEVATOR & MACHINE ROOM TO REMAIN. PROVIDE ACCESS AT ALL TIMES.

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REVISIONS



300 YORK BLVD HAMILTON, ONTARIO L8R 3K6 905-333-9119

35-43 DUKE STAIRS REPLACEMENT

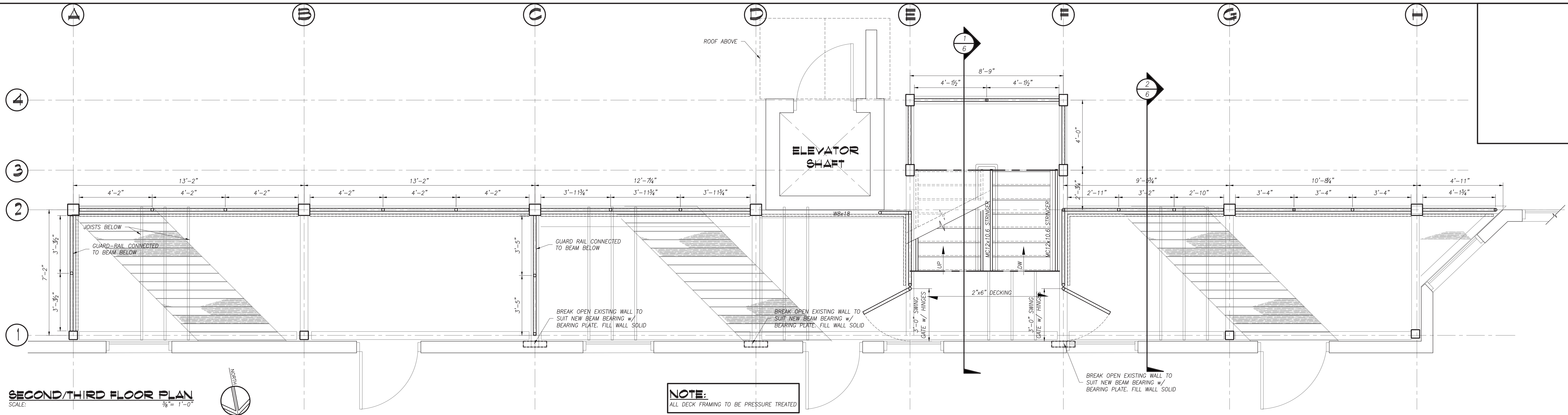
35-43 DUKE STREET

HAMILTON ONTARIO

DEMOLITION PLANS

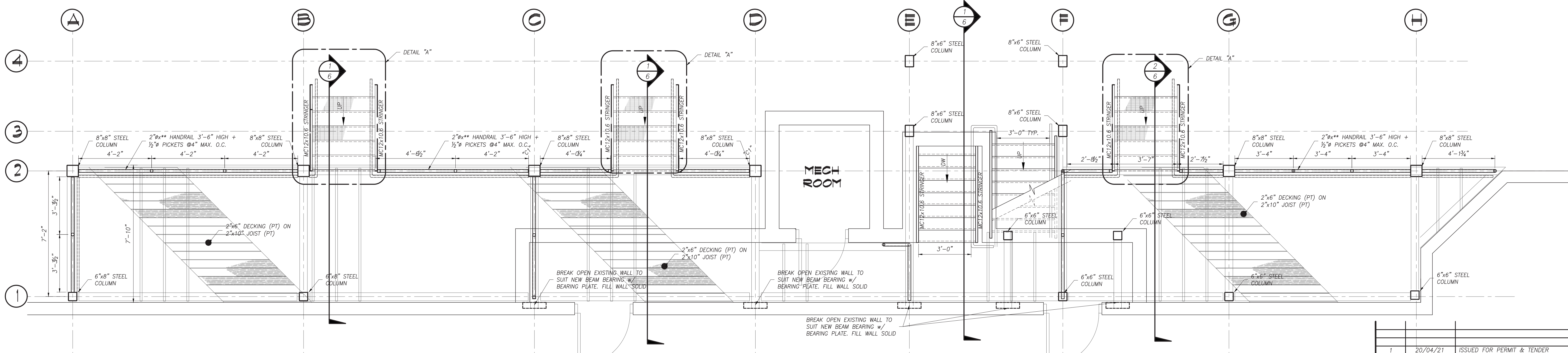
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| DATE APRIL 2020 | DRAWN BY S.NUVE | DRAWING No. 1 OF 7 |
| PROJECT No. 19160 | CHECKED BY HAPH | |





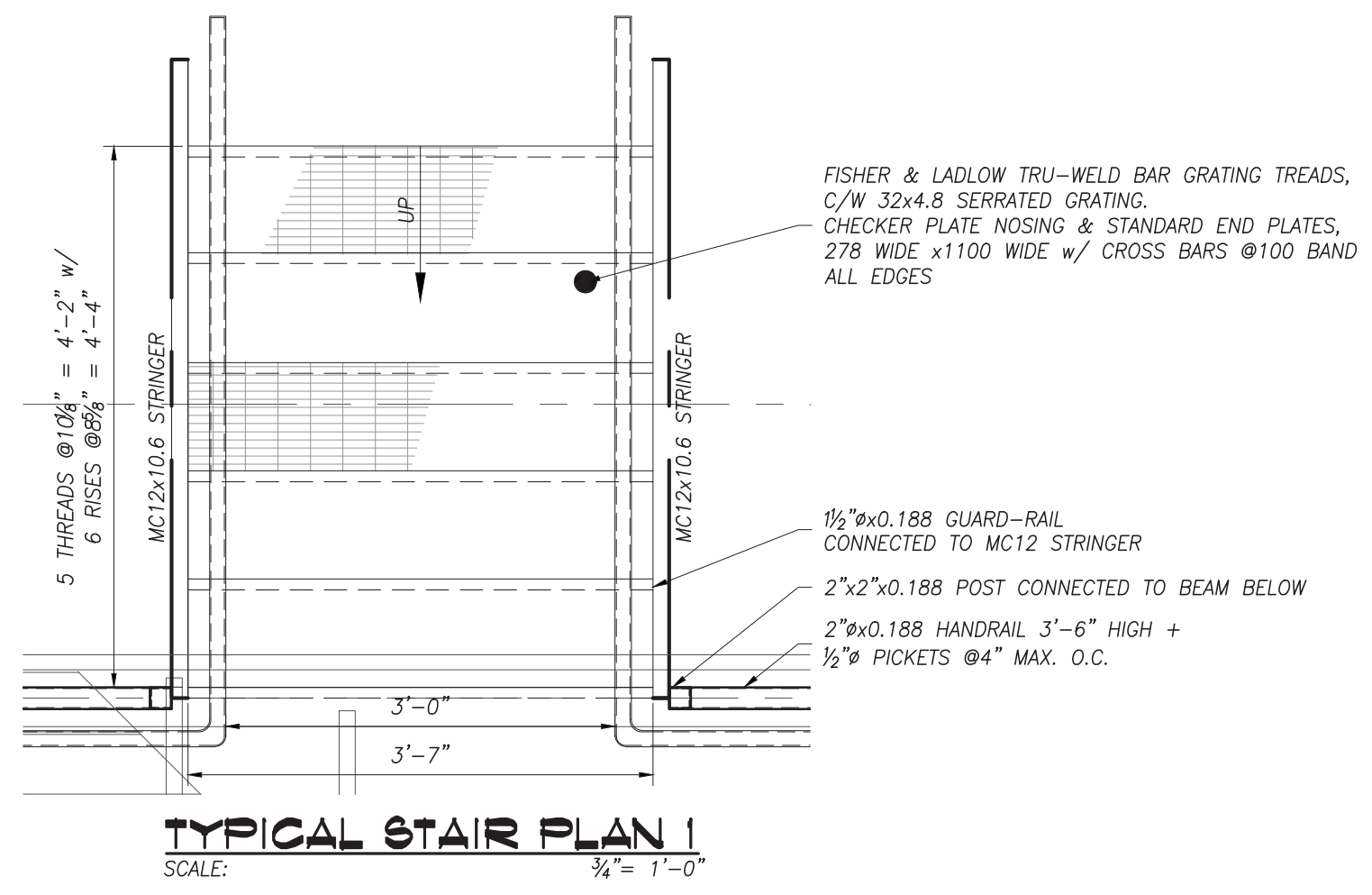
NOTE:
ALL DECK FRAMING TO BE PRESSURE TREATED

SECOND/THIRD FLOOR PLAN
SCALE: 1/8" = 1'-0"



ALTERNATIVE PRICES
PROVIDE SEPARATE PRICES FOR THE FOLLOWING OPTIONS
1. DECKING MATERIAL TO BE 3/4"x6" COMPOSITE
2. DECKING TO BE 1"x6" IPE

MAIN FLOOR PLAN
SCALE: 1/8" = 1'-0"



TYPICAL STAIR PLAN I
SCALE: 3/4" = 1'-0"

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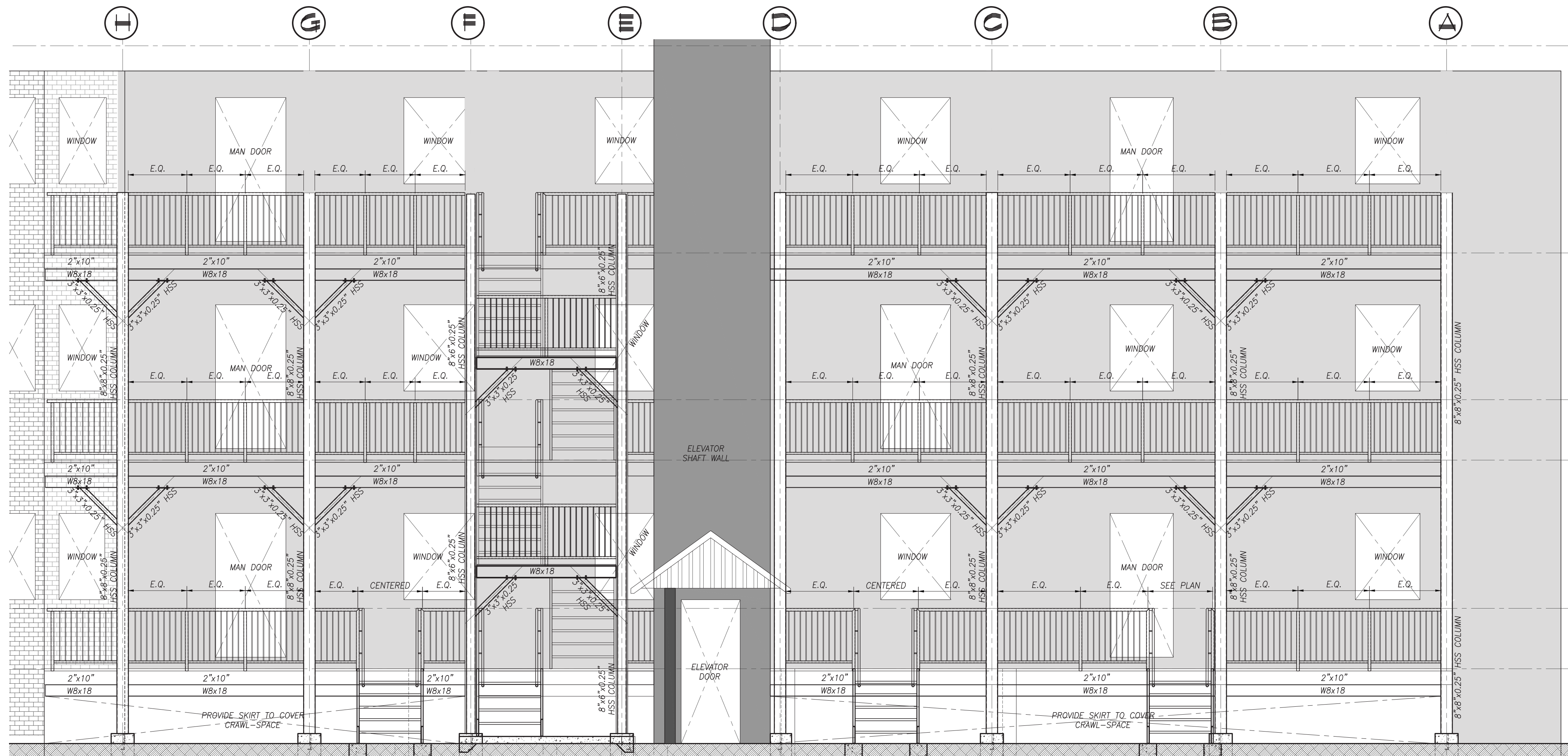
300 YORK BLVD HAMILTON, ONTARIO L8R 3K6 905-333-9119

35-43 DUKE STAIRS REPLACEMENT
35-43 DUKE STREET
HAMILTON ONTARIO

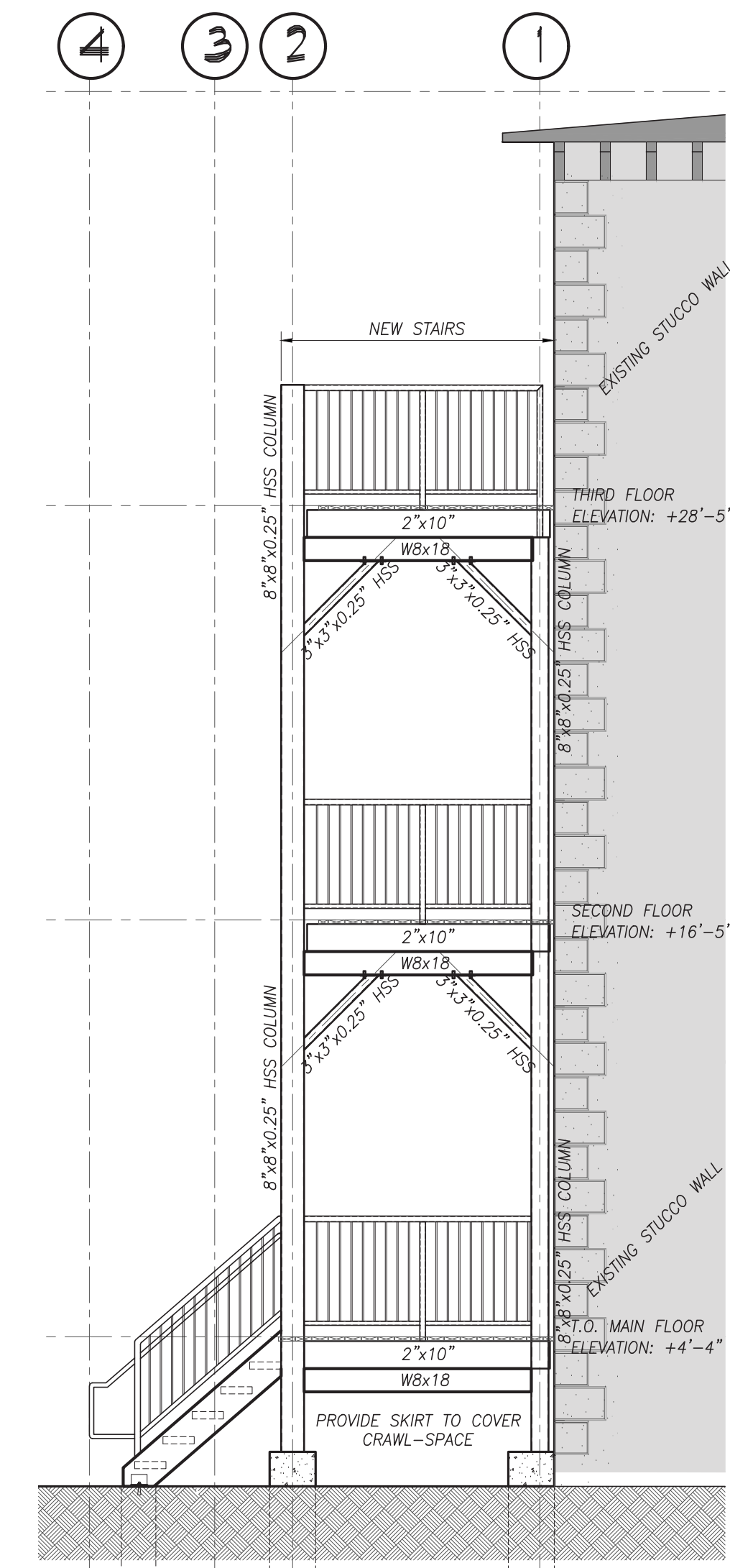
STAIRS PLANS



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STAIR FRONT ELEVATION
SCALE: $\frac{1}{4}'' = 1'-0''$



STAIR SIDE ELEVATION
SCALE: $\frac{1}{4}'' = 1'-0''$

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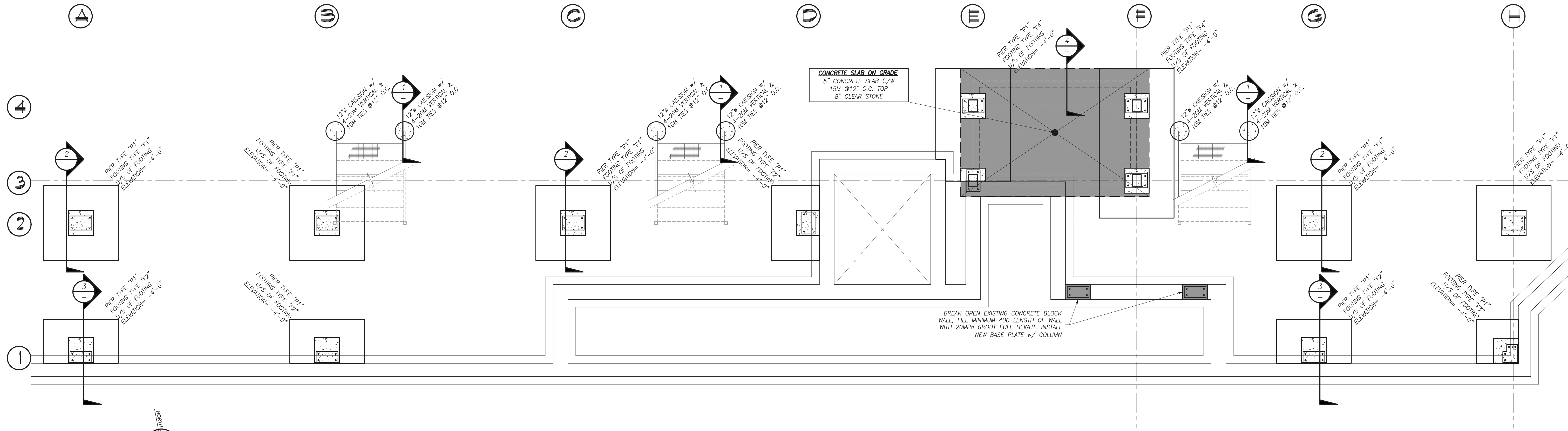
300 YORK BLVD HAMILTON, ONTARIO L8R 3K6 905-333-9119

35-43 DUKE STAIRS REPLACEMENT
35-43 DUKE STREET
HAMILTON ONTARIO

STAIRS ELEVATION

| | | |
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FOUNDATION PLAN
SCALE: 3/8" = 1'-0"

NOTES

- TOP OF STEEL ELEVATION @ MAIN FLOOR +3'-5"
- REFER TO DRAWING 3 FOR NOTES AND 4 DETAILS
- F.M.C. = FULL MOMENT CONNECTION

DESIGN LOAD:

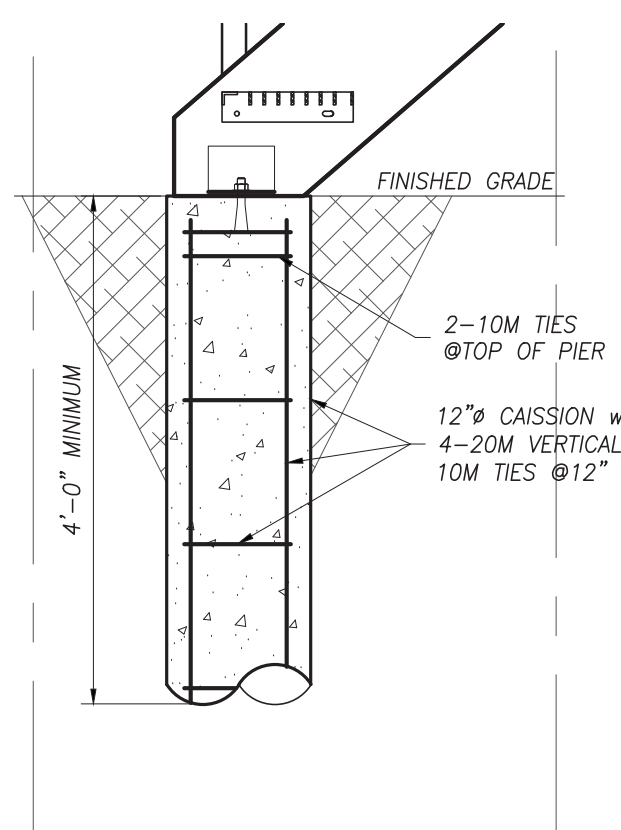
- DEAD LOAD: - 15 Psf
LIVE LOAD: - 100 Psf

STAIR RAILING NOTE:

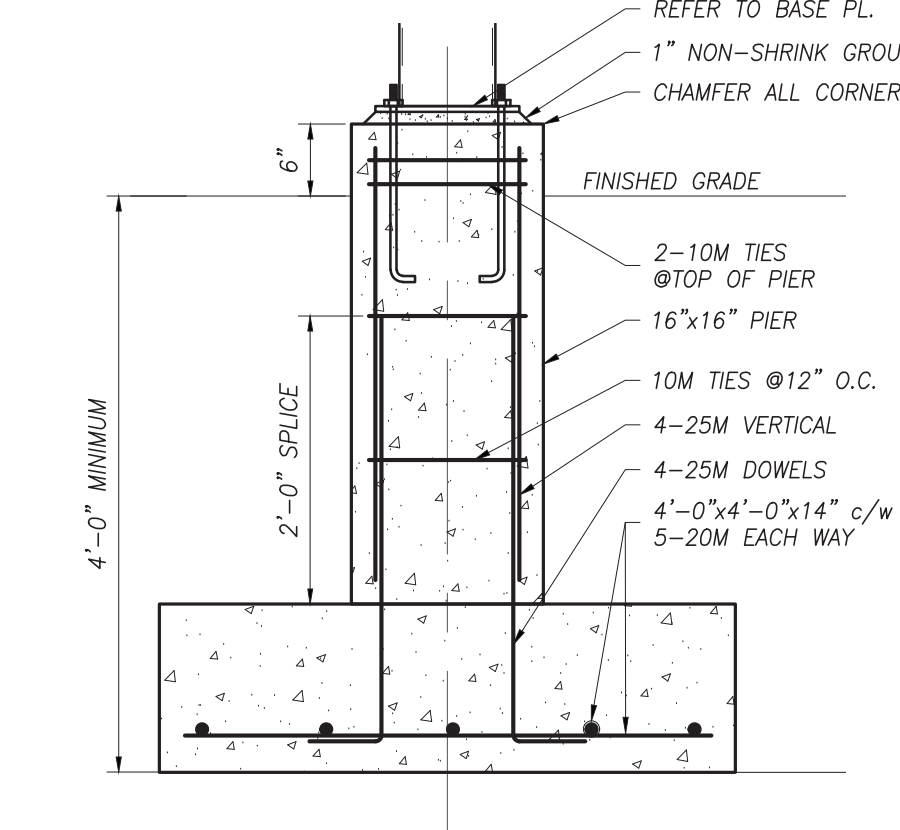
- THE HANDRAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN O.B.C.-2012 DIV. B, ARTICLE 3.4.6.5., 3.4.6.6. & GUARD RAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN ARTICLES 4.1.5.14. & 4.1.5.16. SHOP DRAWINGS ARE TO BE STAMPED BY A PROFESSIONAL ENGINEER

CONCRETE NOTES

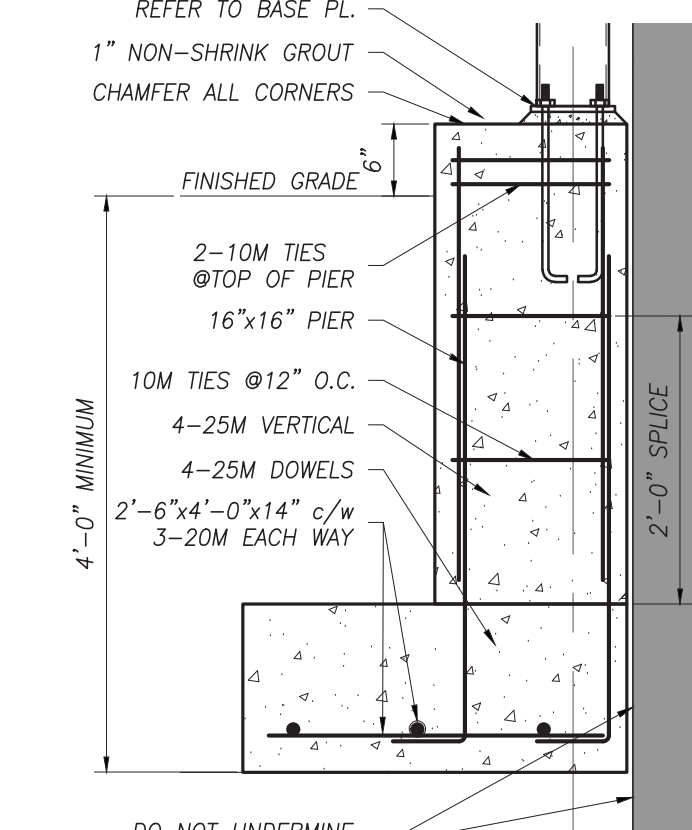
- ALL STRUCTURAL CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.3. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH CSA STANDARD CAN/CSA A23.1.
- MINIMUM CONCRETE STRENGTH AT 28 DAYS SHALL BE:
 - FOOTINGS - 25 MPa TYPE N
 - EXTERIOR SIDEWALK - 32 MPa TYPE C2
 SLUMP SHALL BE 3" ± 1".
AGGREGATE SHALL BE 3/4" MAXIMUM.
AIR ENTRAINMENT TO BE 6% ± 1% WHEN EXPOSED TO EXTERIOR.
CONTRACTOR TO SUBMIT CONCRETE MIX DESIGN FOR REVIEW.
- THE DEFORMED REINFORCING STEEL SHALL CONFORM TO CSA STANDARD G30.18M-09 GRADE 300R FOR STIRRUPS AND TIES AND GRADE 400R FOR ALL OTHER REINFORCING, UNLESS OTHERWISE NOTED THE REINFORCING LAP LENGTH SHALL BE "CLASS B" IN SPLICES. ALL REINFORCING HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH A23.1.
- WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH CSA G30.5. ALL MESH SHALL BE CHAIRED PRIOR TO THE CONCRETE POUR. LIFTING OF THE MESH DURING THE CONCRETE POUR WILL NOT BE PERMITTED. ALL SPLICES SHALL BE A MINIMUM OF TWO CROSSWIRE SPACINGS PLUS 2".
- THE REINFORCING COVER FOR CONCRETE SHALL BE:
 - 3" FOR CONCRETE AGAINST EARTH
 - 1 1/2" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 15M OR SMALLER
 - 2" FOR FORMED CONCRETE EXPOSED TO EARTH OR WEATHER WHERE THE REINFORCING BAR IS 20M OR LARGER
 - 1" FOR INTERIOR CONCRETE, ALL CHAIRS, BOLSTERS, SPACERS AND BAR SUPPORTS SHALL BE IN ACCORDANCE WITH A23.1.
- FOOTINGS SHALL BEAR ON NATIVE UNDISTURBED SOIL OR ENGINEERED FILL WITH A MINIMUM BEARING RESISTANCE OF:
 - 2500 psf (SLS)
 - 3000 psf (ULS)
 THE CONTRACTOR SHALL VERIFY THE CAPACITY PRIOR TO PLACEMENT OF CONCRETE.
- THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATION OR STEP DOWN FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10. STEP HEIGHT SHALL NOT EXCEED 2'-0".
- KEEP EXCAVATIONS DRY BEFORE CONCRETE IS PLACED. REMOVE ALL LOOSE MATERIAL, SOFT SOIL OR WATER PRIOR TO PLACING CONCRETE. PROVIDE A 3" MUD MAT FOR ALL FOOTINGS BELOW THE WATER TABLE.
- ALL FOOTINGS SHALL BE CENTERED ON THE WALL UNLESS OTHERWISE NOTED.
- THE FOOTING DESIGN IS BASED ON INFORMATION AVAILABLE AT THE TIME OF DESIGN. THE FOOTING DESIGN MAY BE ALTERED DURING CONSTRUCTION, IF THE SITE CONDITIONS WARRANT, BUT ONLY WITH THE EXPRESS PERMISSION OF THE ENGINEER.
- PROTECT ALL FOOTINGS, WALLS AND SLABS AGAINST FROST ACTION DURING CONSTRUCTION. ALL EXTERIOR FOOTINGS SHALL BE FOUNDED BELOW THE FROST LINE, MINIMUM 5'-0" BELOW GRADE.
- DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL THE ELEMENTS PROVIDING LATERAL SUPPORT ARE COMPLETE. PLACE BACKFILL IN A MANNER WHERE THE ELEVATION DIFFERENCE ON EITHER SIDE OF THE WALL IS NO GREATER THAN 1'-6". PROVIDE TEMPORARY SHORING AS REQUIRED.
- SLAB-ON-GRADE GRADE CONSTRUCTION SHALL BE CAPABLE OF SUPPORTING 500 lbs/ft² WITHOUT RELATIVE SETTLEMENT.
- CONSTRUCT CONCRETE WALLS WITHOUT CONTROL JOINTS, UNLESS OTHERWISE NOTED. PROVIDE CHASES AND BEAMS POCKETS IN THE INTERIOR FACE OF THE WALL AS REQUIRED.
- PROVIDE DOWELS TO WALLS AND COLUMNS TO SUIT THE REINFORCING IN THE WALL OR COLUMN ABOVE.
- ALL ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE Hilti HIT-HY200 (OR APPROVED EQUAL) PROCEDURES.



SECTION 1
SCALE: 3/4" = 1'-0"



SECTION 2
SCALE: 3/4" = 1'-0"

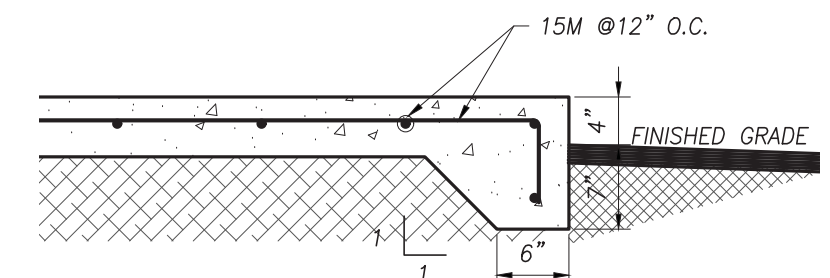


SECTION 3
SCALE: 3/4" = 1'-0"

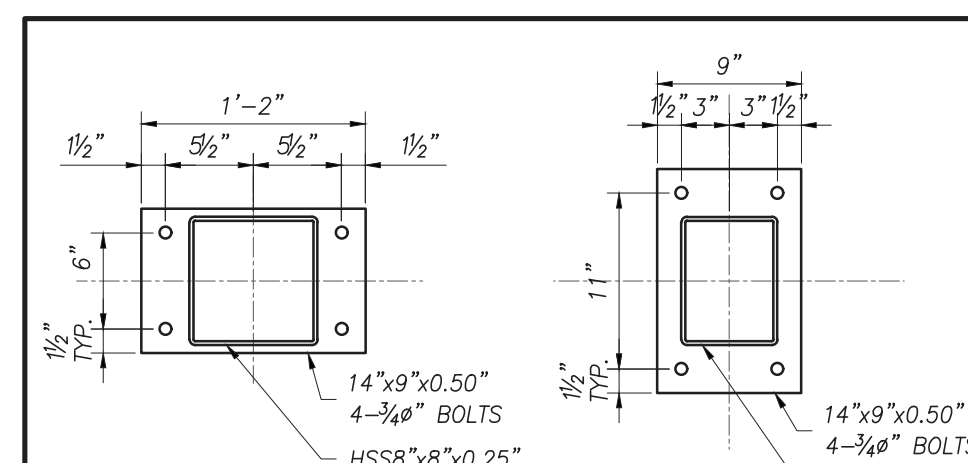
| PIER SCHEDULE | | | |
|---------------|---------|---|---------------------|
| TYPE | SIZE | REINFORCING | T.O. PIER ELEVATION |
| P1 | 16"x16" | 4-25M VERTICAL w/ 10M TIES @12" O.C. | +150 |

| FOOTING SCHEDULE | | | |
|------------------|-----------------|-----------------------------|-----------------------|
| TYPE | SIZE | REINFORCING | U/S FOOTING ELEVATION |
| F1 | 4'-0"x4'-0"x14" | 5-20M EACH WAY | -4'-0" |
| F2 | 2'-6"x4'-0"x16" | 4-20M SHORT & 6-20M LONG | -4'-0" |
| F3 | 2'-6"x2'-6"x16" | 3-20M EACH WAY | -4'-0" |
| F4 | 8'-0"x4'-0"x14" | 10-20M EACH WAY | -4'-0" |

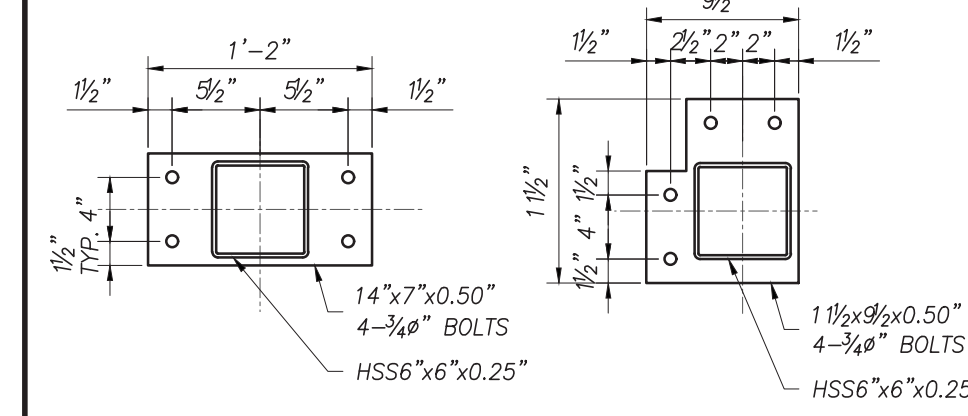
| COLUMN SCHEDULE | | | |
|-----------------|-------------|--------------|-----------|
| TYPE | SIZE | BASE PLATE | CAP PLATE |
| C1 | 8"x8"x0.25" | BASE PLATE 1 | 6mm |
| C1 | 8"x6"x0.25" | BASE PLATE 2 | 6mm |
| C1 | 6"x6"x0.25" | BASE PLATE 3 | 6mm |



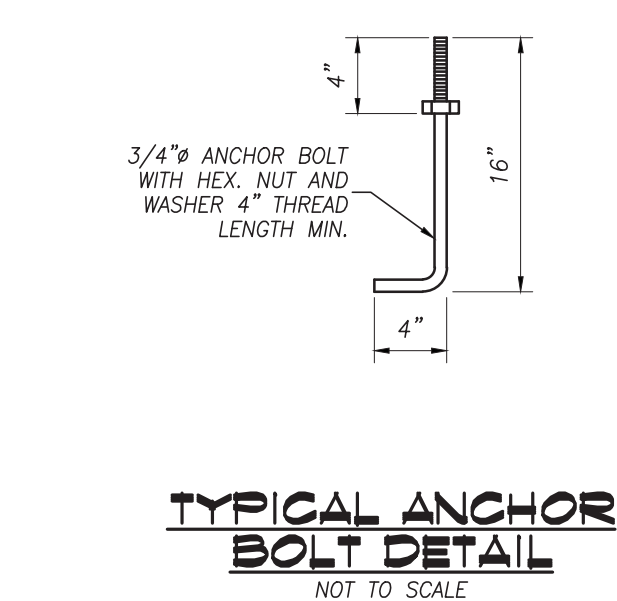
SECTION 4
SCALE: 3/4" = 1'-0"



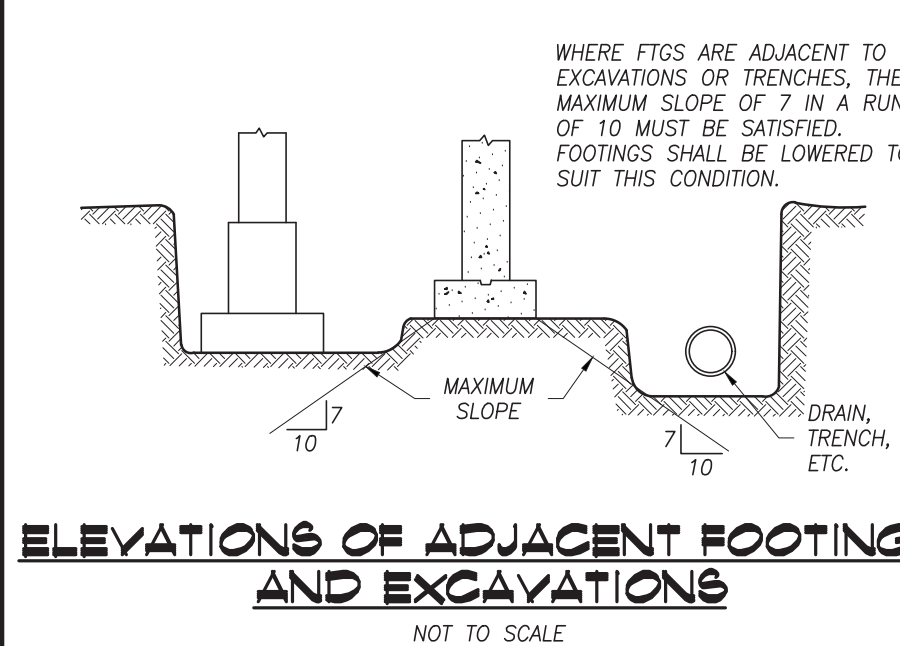
BASE PLATE "A" SCALE: 1" = 1'-0"
BASE PLATE "B" SCALE: 1" = 1'-0"



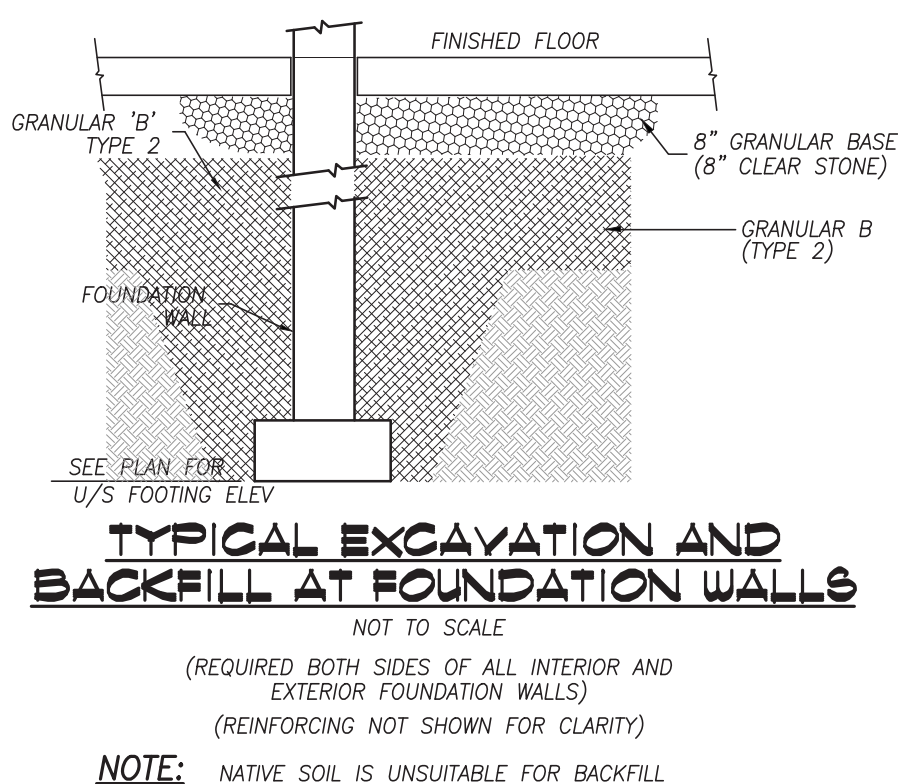
BASE PLATE "C" SCALE: 1" = 1'-0"
BASE PLATE "D" SCALE: 1" = 1'-0"



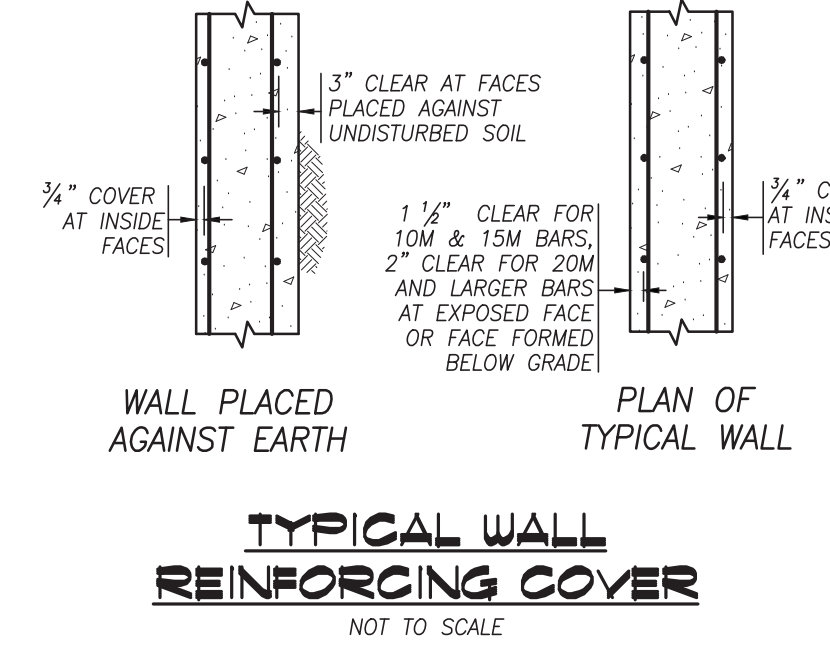
TYPICAL ANCHOR BOLT DETAIL
NOT TO SCALE



ELEVATIONS OF ADJACENT FOOTINGS AND EXCAVATIONS
NOT TO SCALE



TYPICAL EXCAVATION AND BACKFILL AT FOUNDATION WALLS
NOT TO SCALE
(REQUIRED BOTH SIDES OF ALL INTERIOR AND EXTERIOR FOUNDATION WALLS)
(REINFORCING NOT SHOWN FOR CLARITY)
NOTE: NATIVE SOIL IS UNSUITABLE FOR BACKFILL



TYPICAL WALL REINFORCING COVER
NOT TO SCALE

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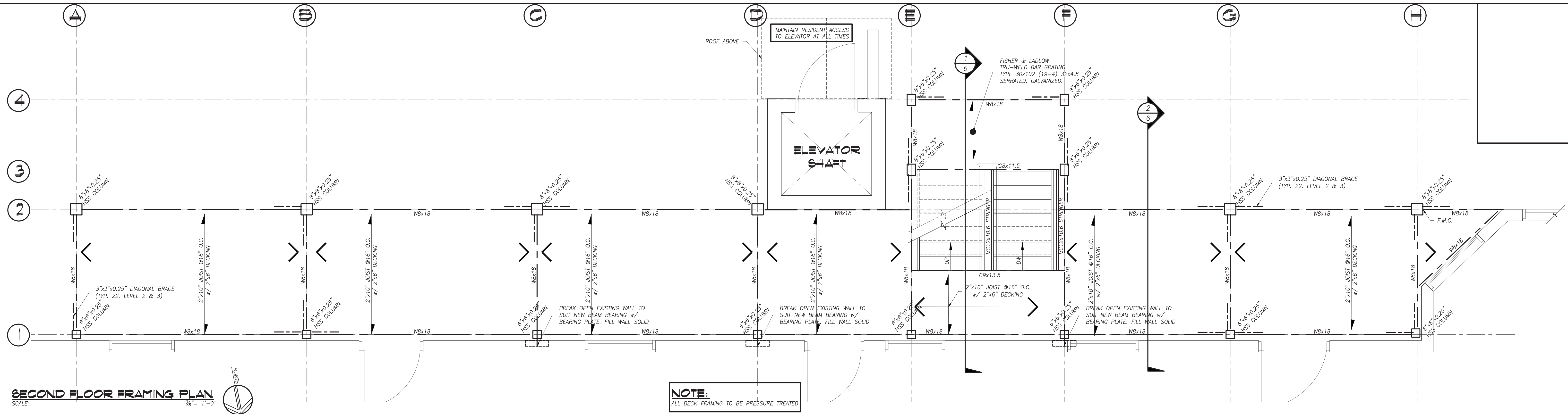
300 YORK BLVD HAMILTON, ONTARIO L8R 3K6 905-333-9119

35-43 DUKE STAIRS REPLACEMENT
35-43 DUKE STREET

FOUNDATION PLAN, SECTIONS & NOTES

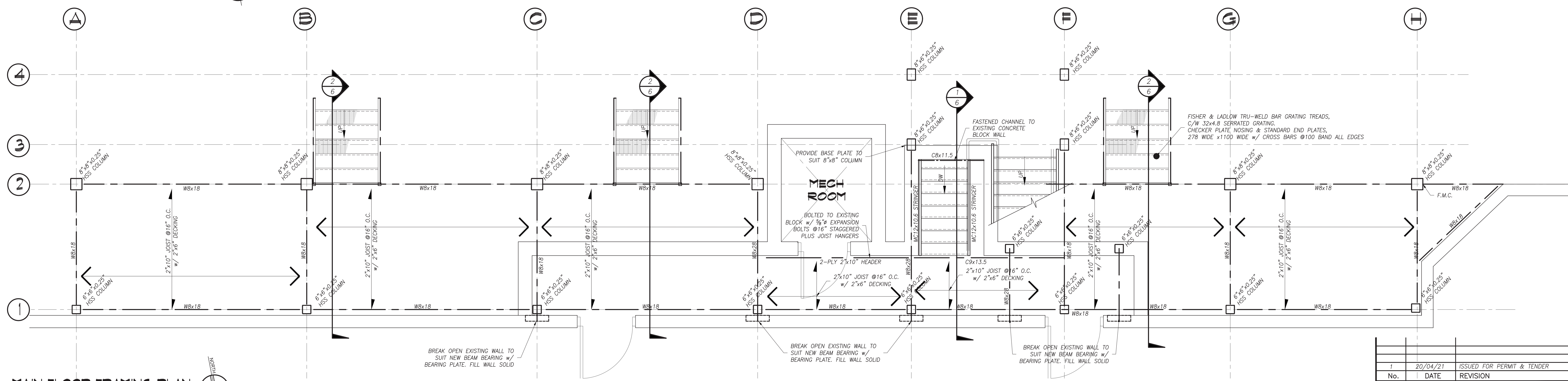
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SECOND FLOOR FRAMING PLAN
SCALE: 3/8" = 1'-0"

NOTE:
ALL DECK FRAMING TO BE PRESSURE TREATED



MAIN FLOOR FRAMING PLAN
SCALE: 3/8" = 1'-0"

- NOTES**
1. TOP OF STEEL ELEVATION @THIRD FLOOR +27'-6"
 2. TOP OF STEEL ELEVATION @SECOND FLOOR +15'-6"
 3. REFER TO DRAWING S3 FOR NOTES AND S4 FOR DETAILS

DESIGN LOAD:

DEAD LOAD: - 15 Psf
LIVE LOAD: - 100 Psf

STAIR RAILING NOTE:

1. THE HANDRAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN O.B.C.-2012 DIV. B. ARTICLE 3.4.6.5., 3.4.6.6. & GUARD RAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN ARTICLES 4.1.5.14. & 4.1.5.16. SHOP DRAWINGS ARE TO BE STAMPED BY A PROFESSIONAL ENGINEER

TIMBER FRAMING

1. ALL DESIGN AND CONSTRUCTION SHALL BE CARRIED OUT IN CONFORMANCE WITH CSA STANDARD O86.
2. SAWN LUMBER SHALL BE SPRUCE/PINE/FIR GRADE NUMBER 1 AND GRADE NUMBER 2 IN THE RATIO OF 67% (MINIMUM) AND 33% (MAXIMUM) RESPECTIVELY. PRESSURE TREATED.
3. ALL TIMBER EXPOSED TO EXTERIOR SHALL BE PRESSURE TREATED.

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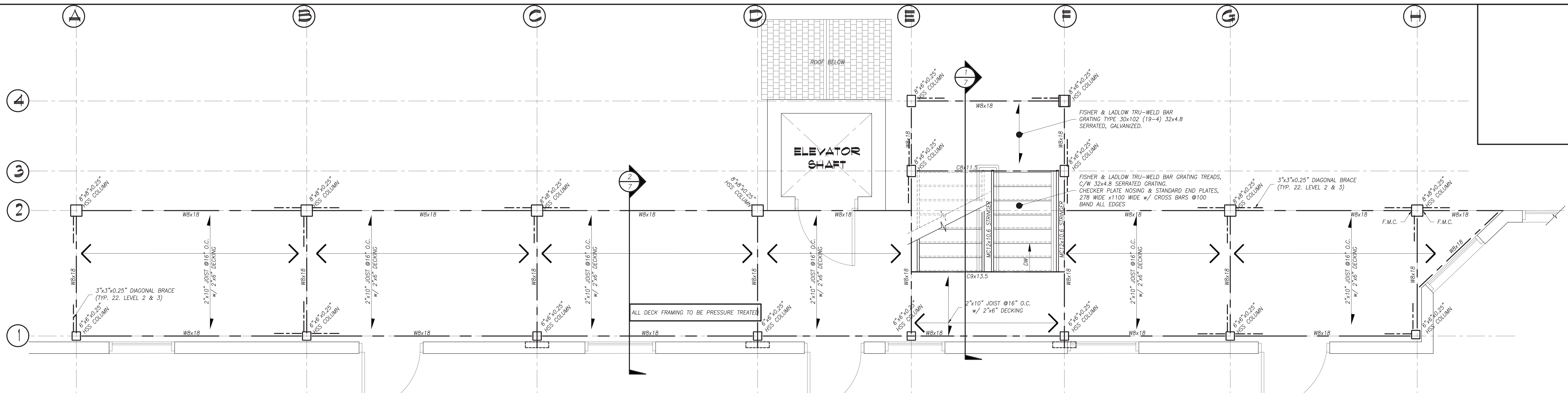
35-43 DUKE STAIRS REPLACEMENT
35-43 DUKE STREET
HAMILTON ONTARIO

3RD & 4TH FLOOR FRAMING PLAN & NOTES

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THIRD FLOOR FRAMING PLAN
SCALE: 3/8" = 1'-0"

NOTES

1. TOP OF STEEL ELEVATION @THIRD FLOOR +27'-6"
2. TOP OF STEEL ELEVATION @SECOND FLOOR +15'-6"
3. REFER TO DRAWING S3 FOR NOTES AND S4 FOR DETAILS

DESIGN LOAD:

DEAD LOAD: - 15 Psf
LIVE LOAD: - 100 Psf

STAIR RAILING NOTE:

1. THE HANDRAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN O.B.C.-2012 DIV. B, ARTICLE 3.4.6.5., 3.4.6.6. & GUARD RAILS MUST BE DESIGNED TO WITHSTAND LOADING AS SPECIFIED IN ARTICLES 4.1.5.14. & 4.1.5.16. SHOP DRAWINGS ARE TO BE STAMPED BY A PROFESSIONAL ENGINEER

TIMBER FRAMING

1. ALL DESIGN AND CONSTRUCTION SHALL BE CARRIED OUT IN CONFORMANCE WITH CSA STANDARD O86.
2. SAWN LUMBER SHALL BE SPRUCE/PINE/FIR GRADE NUMBER 1 AND GRADE NUMBER 2 IN THE RATIO OF 67% (MINIMUM) AND 33% (MAXIMUM) RESPECTIVELY. PRESSURE TREATED.
3. ALL TIMBER EXPOSED TO EXTERIOR SHALL BE PRESSURE TREATED.

GENERAL NOTES

1. CHECK ALL DIMENSIONS ON THESE DRAWINGS WITH ALL OTHER DRAWINGS, INCLUDING BUT NOT LIMITED TO DRAWINGS PREPARED ARCHITECTURAL, MECHANICAL OR ELECTRICAL CONSULTANTS. REPORT ANY INCONSISTENCIES TO THE ENGINEER PRIOR TO COMMENCING WITH THE WORK. DO NOT SCALE THE DRAWINGS.
2. THE DESIGN LIVE LOADS ARE INDICATED ON THE DRAWINGS. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LOADS.
3. THE COMPLETED STRUCTURE IS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, SHORING AND ANY OTHER TEMPORARY OR PERMANENT MEASURES AS REQUIRED DURING CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY SUPPORT OF EXISTING OR ADJACENT STRUCTURES AS REQUIRED. ALL BRACING AND SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
4. CONSTRUCTION FEATURES NOT FULLY SHOWN ARE COMPARABLE TO SIMILAR CONDITION DETAILS.
5. REFER TO OTHER CONSULTANTS DRAWINGS FOR DETAILS OF OPENINGS, PITS, CHAMFERS, DEPRESSIONS NOT INDICATED ON THE STRUCTURAL DRAWINGS.
6. ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE LATEST ONTARIO BUILDING CODE, LATEST APPLICABLE REGULATIONS AND GOOD CONSTRUCTION PRACTICES.
7. THE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS.
8. CLARIFY ANY QUERIES WITH THE ENGINEER REGARDING THE INTERPRETATION OF THE DRAWINGS, PRIOR TO THE COMMENCEMENT OF ANY WORK.

SUBMITTALS

1. SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: CONCRETE FORMWORK, REINFORCING STEEL, STRUCTURAL STEEL AND TEMPORARY SHORING.
2. THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN 1/4"=1'-0" (1:50).
3. THE STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED, IN WHOLE OR IN PART, FOR USE AS SHOP DRAWINGS.
4. EACH DRAWING SUBMITTED FOR CONCRETE FORMWORK, STRUCTURAL STEEL AND TEMPORARY SHORING SHALL BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
5. CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.

CALCULATIONS

1. SUBMIT CALCULATIONS, BEARING THE SEAL AND SIGNATURE OF PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, FOR STRUCTURAL WORK, IF REQUESTED BY THE CONSULTANT.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL ELEMENTS, INCLUDING DESIGN OF ELEMENTS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH CAN/CSA S16.
2. ALL STRUCTURAL STEEL SHALL CONFORM TO CSA G40.21 (300W) EXCEPT W SECTIONS AND PLATES G40.21 (350W), HSS MEMBERS G40.21 (350W) CLASS C OR ASTM A500 GRADE C, ANCHOR BOLTS ASTM A307, COLD FORM SECTIONS ASTM A570M GRADE 350W. UNLESS OTHERWISE NOTED, ALL SECTIONS SHALL BE PRIME PAINTED WITH THE SURFACE PREPARATION AND PAINTING PROCEDURES IN ACCORDANCE WITH CAN/COSB 85.10.
3. ALL WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH CAN/CSA W59. THE STEEL FABRICATOR SHALL BE FULLY QUALIFIED UNDER THE REQUIREMENTS BY THE CANADIAN WELDING BUREAU IN CONFORMANCE WITH CAN/CSA W47.1.
4. DESIGN ALL MOMENT AND SHEAR CONNECTIONS FOR THE FULL CAPACITY OF THE SMALLER MEMBER IN THE CONNECTION UNLESS OTHERWISE NOTED.
5. PROVIDE MINIMUM BEARING LENGTH OF STEEL MEMBERS AS FOLLOWS:
- ON MASONRY - 6"
- ON STEEL - 4"
6. THE BASE PLATE AND BEARING PLATE GROUT SHALL BE OF THE CEMENTITIOUS NON-SHRINK TYPE.
7. FULLY WELD THE BASE PLATE TO THE COLUMN TO DEVELOP THE ANCHOR BOLTS. PROVIDE CAP PLATES ON ALL COLUMNS. PROVIDE 1/4" CAP PLATES ON ALL COLUMNS.
8. PROVIDE MINIMUM 7"x3/8"x7" BEARING PLATES FOR ALL STRUCTURAL STEEL c/w 2-3/8" ANCHORS UNLESS OTHERWISE NOTED.
9. ALL BOLTS SHALL BE TIGHTENED WITH A SUITABLE TORQUE WRENCH IN ACCORDANCE WITH CSA S16.
10. ALL STEEL EXPOSED TO THE EXTERIOR TO BE HOT DIP GALVANIZED.
11. ERECT STRUCTURAL STEEL IN ACCORDANCE WITH CSA S16 AND IN CONFORMANCE WITH THE APPROVED SHOP DRAWINGS.

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35-43 DUKE STAIRS REPLACEMENT

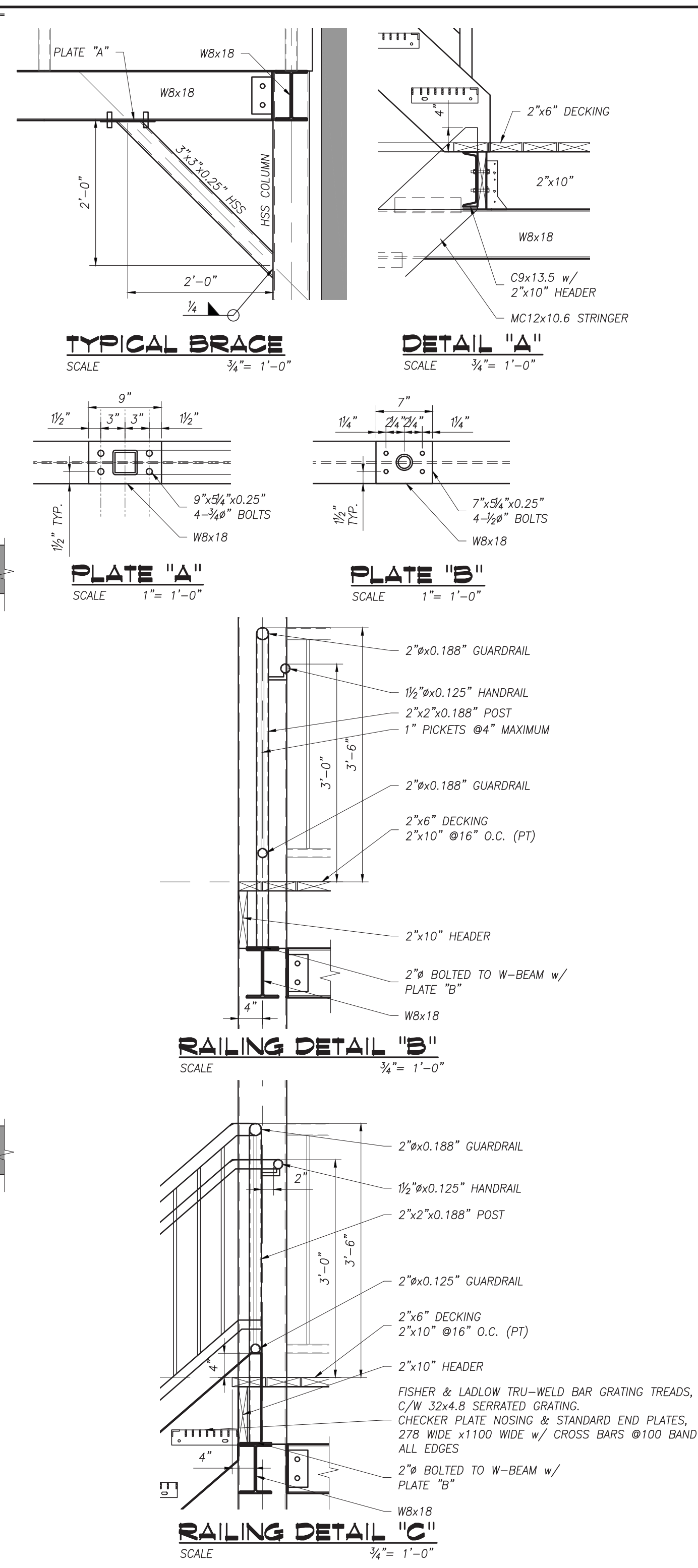
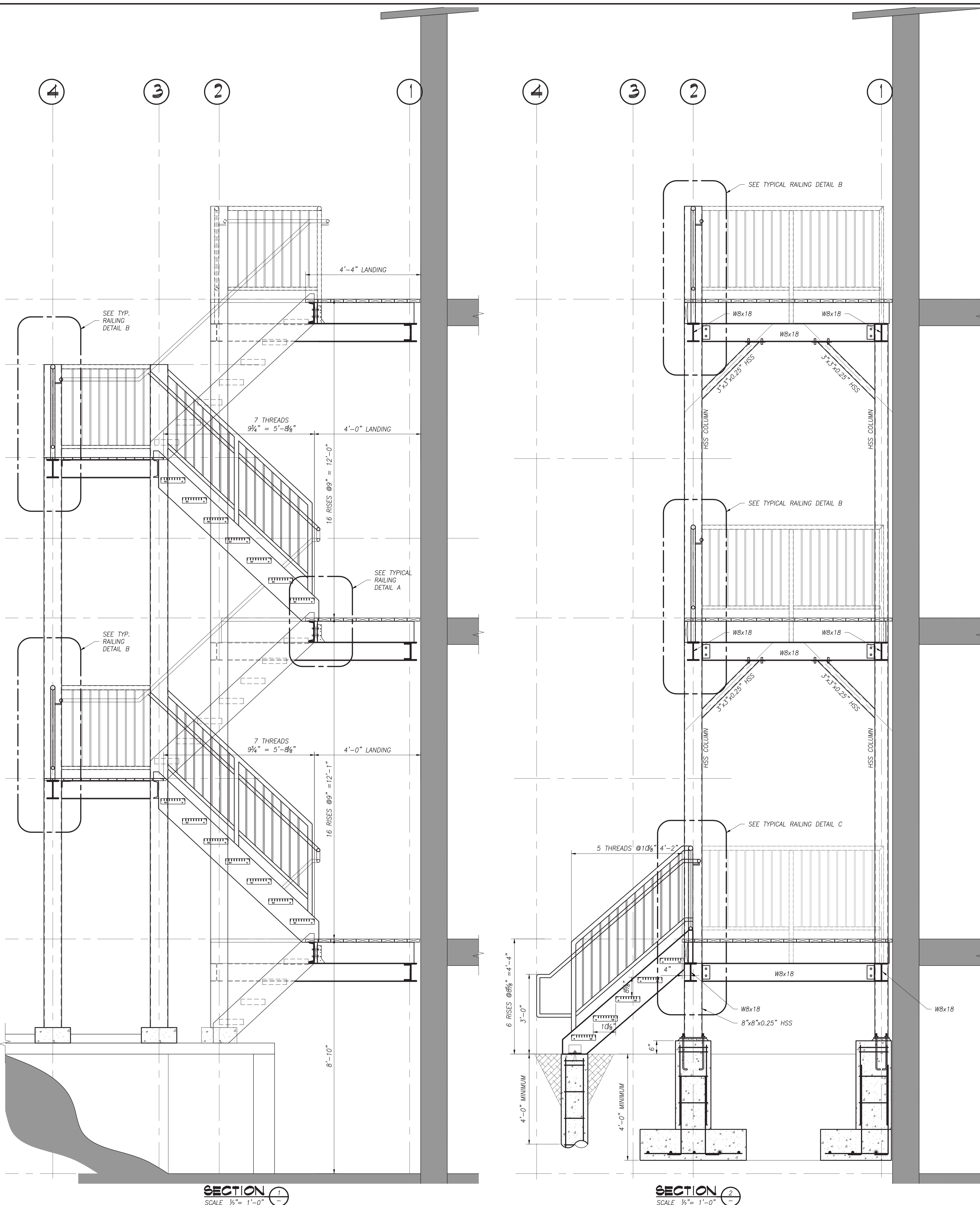
35-43 DUKE STREET
HAMILTON ONTARIO

3RD FLOOR FRAMING PLAN & NOTES



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300 YORK BLVD HAMILTON, ONTARIO L8R 3K6 905-333-9119

35-43 DUKE STAIRS REPLACEMENT

35-43 DUKE STREET

HAMILTON ONTARIO

SECTIONS & DETAILS

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| PROJECT No. 19160 | CHECKED BY HAPH | |

